

# REVIEW MODULE - REINFORCED CONCRETE DESIGN (COLUMNS)



## CIRCULAR SECTIONS

$$r = \sqrt{\frac{I}{A}}$$

$$P_u = 1.4 P_{DL} + 1.7 P_{LL}$$

NSCP 404

$$r = \sqrt{\frac{\frac{\pi}{4} D^4}{\frac{\pi}{4} D^2}}$$

where:  
 $P_u$  = factored load.  
 $P_{DL}$  = dead axial load.  
 $P_{LL}$  = live axial load.

$$r = \sqrt{\frac{D^2}{16}}$$

- \* NOTE:  
 1. FOR DIMENSIONS OF COLUMN, ROUND UP.  
 2. FOR NUMBER OF STEEL BARS, ROUND UP.

$$r = \frac{D}{4} = 0.25D$$

$$r > 0.25D \rightarrow$$

- NSCP  
 3. FOR SPACING OF BARS, ROUND DOWN.

## COLUMNS

### ULTIMATE STRENGTH DESIGN

#### NSCP PROVISIONS

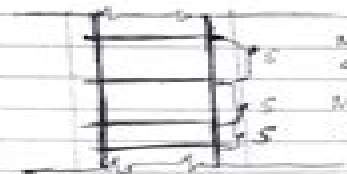
#### I. SHORT COLUMNS (Axially Loaded Columns)

$$a. \frac{KL_u}{r} \leq 12 \rightarrow \frac{M_u}{P_u} \leq 0.15 \quad b_{\text{eff}} \geq 4b_{\text{min}}$$

#### II. LONG COLUMNS (Eccentrically Loaded Columns)

$$a. \frac{KL_u}{r} > 12 \rightarrow \frac{M_u}{P_u} \leq 0.15 \quad b_{\text{eff}} \geq 4b_{\text{min}}$$

#### SHORT TIED COLUMNS



Min. concrete cover = 40mm  
 Min. steel cover = 45mm

\* NOTE:  
 1. Design strength  $\phi P_n$  and ultimate load  $P_u$ .  
 Tied Columns:  $\phi = 0.65$   
 Spiral Columns:  $\phi = 0.75$

\* NOTE:  
 (Ties)  $\phi \geq 10$   
 (Spiral)  $\phi \geq 8$

## Without End-Moments

$$\frac{KL_u}{r} \leq 22$$

## Eccentrically Loaded Column

$$\frac{KL_u}{r} > 22$$

where:

$K$  = effective length factor.  
 $L_u$  = unbraced / unsupported length.  
 $r$  = radius of gyration.

$$r = \sqrt{\frac{I}{A}}$$

## RECTANGULAR SECTIONS

$$I = \frac{hb^3}{12} \quad ; \quad A = hb$$

$h$  = larger dimension.

$b$  = smaller dimension.

$$r = \sqrt{\frac{Kh^3/12}{Kh}}$$

$$r = \sqrt{\frac{b^2}{12}}$$

$$r = b \sqrt{\frac{1}{12}} \quad \text{or} \quad 0.289b$$

$$r = 0.3b \rightarrow \text{NSCP}$$

# Harrison Study Guide For Civil Engineering

**Christian G. Meyer**



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